

ENGINEERING CALCULATION DETAIL SHEET

Outdoor Condensor Units on Wall Bracket - Suitability Verification
Designed by: Paul C. Perrin, PE, SE

DESIGN METHODOLOGY: ASD

OBJECTIVE:

Determine Wind Load on AC unit mounted on wall bracket using ASCE 7 (2010), Section 29.5. Confirm stability, wall bracket strength, anchor configuration and strength, and equipment envelope fastening.

WIND LOAD: (See also "Wind Design Requirements" on Sheet 1)

Vult = 186 mph (FBC 2014 1620.2) for Miami-Dade, Risk Category IV

From "29.3 Velocity Pressure" for Building Height = 100'
 $qz = 0.00256 * Kz * Kzt * Kd * V^2 = 114.0 \text{ psf}$ (Eq. 29.3-1)
From ASCE 7 Chapter 30 "Wind Loads - Components & Cladding"
Table 30-6.1 for Building h > 60 ft.
 $F = qz(GCp)Af$
Fvertical (uplift) = 114.0 psf * (2.3) * Af = 262 psf x Area (ft²)
Fvertical (down) = 114.0 psf * (0.9) * Af = 103 psf x Area (ft²)
Flateral = 114.0 psf * (0.9) * Af = 103 psf x Area (ft²)

Example AC Unit:

Use LUU187HV in Table w/ dims (W, D, H, Wt)=(37.41", 13.00", 32.84", 132.3 lbs)

Example Anchors:

5/8" HILTI KWIK BOLT 3 with 3 1/2" embed with capacity of combined pullout load of 2400# and shear load of 300#.

WIND LOAD FORCES:

Top Area = 37.41" * 13.00" / (144 in²/ft²) = 3.38 sf
Fw vertical (Fw_vert) = 262 psf * 3.38 sf = 885 lbs (unfactored)
Fw vertical downward (Fw_vert_down) = 103 psf * 3.38sf = 346 lbs (unfactored)
Long Side Area = 37.41" * 32.84" / (144 in²/ft²) = 8.53 sf
Fw lateral (Fw_lat_out) = 103 psf * 8.53 sf = 875 lbs (unfactored)
Short Side Area = 13.00" * 32.84" / (144 in²/ft²) = 2.97 sf
Fw lateral (Fw_lat_edge) = 103 psf * 2.97 sf = 304 lbs (unfactored)

LOAD COMBINATIONS:

0.67D + 0.78W for overturning FBC 1605.3.2 Eq. 16-18
0.6D + 0.6W for sliding and anchors FBC 1605.3.1 Eq. 16-15

VERIFY BOLT SHEAR RESISTANCE TO AC UNIT SLIDING:

Shear Vbolt = 0.6 * √ (Fw_lat_out² + Fw_lat_edge²) = 556 lbs
Rn / Ω = 663# * 4 bolts = 2652 lbs > 556 lbs

Resistance to Bolt Shear Checks OK

VERIFY WALL ANCHOR SHEAR RESISTANCE:

Shear Vwall = 0.6 * √ [(Fw_vert - Wt)² + Fw_lat_edge²] = 487 lbs
Wall Anchor Shear Capacity = 300# * 4 bolts = 1200 lbs > 487 lbs

Resistance to Wall Anchor Shear Checks OK

CHECK ANCHOR PULLOUT FROM MOMENT ABOUT BOTTOM OF BRACKET:

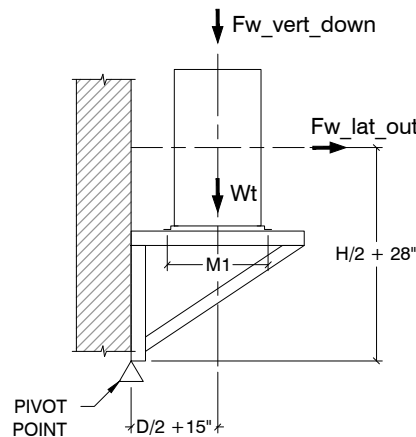
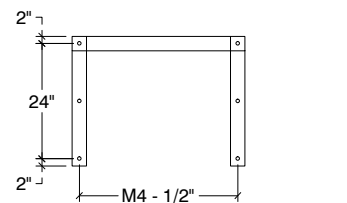
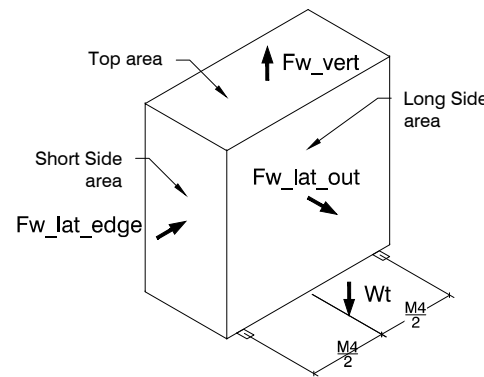
Consider downward vertical wind load with equal pressure to lateral suction wind load on wall.

Moment Mbot =
= 0.78 * Fw_lat_out * (H/2 + 24" + 4") + (0.67 * Wt + 0.78 * Fw_vert_down) * (D/2 + 15")
= 37.36 kip*in

Wall Anchor Pullout Capacity = 2400 lbs > Mbot / [2 anchors * (24" + 2")] = 718 lbs
Resistance to Wall Anchor Pullout Checks OK

THE CALCULATIONS ON THE DRAWING ARE REPRESENTATIVE OF THE FOLLOWING LG ELECTRONICS OUTDOOR CONDENSING UNITS:

LMU30CHV
LMU36CHV
LUU187HV
LUU247HV
LMU480HV
LMU540HV
LMU600HV
LUU367HV
LUU427HV
ARUN038GSS4
ARUN048GSS4
ARUN054GSS4



SINCE THIS DESIGN IS BASED ON WIND PRESSURE, qz, THIS DESIGN IS ALSO SUITABLE FOR THE FOLLOWING CASES:

- MIAMI DADE WIND SPEED = 186 MPH, RISK CATEGORY IV, EXPOSURE CATEGORY C, HEIGHT UP TO 180 FT.
- MIAMI DADE WIND SPEED = 186 MPH, RISK CATEGORY II, EXPOSURE CATEGORY D, HEIGHT UP TO 200 FT.
- BROWARD WIND SPEED = 180 MPH, RISK CATEGORY IV, EXPOSURE CATEGORY D, HEIGHT UP TO 140 FT.

THIS DESIGN IS FOR HVAC UNITS AND WALL BRACKETS IN WALL WIND ZONE 4. HVAC UNITS AND WALL BRACKETS SHALL NOT BE WITHIN 3 FEET OF ANY BUILDING CORNER OR TOP OF WALL. FOR THIS PURPOSE, A BUILDING CORNER IS DEFINED AS ANYWHERE TWO EXTERIOR WALLS INTERSECT.

DESIGN METHODOLOGY: ASD

CHECK ANCHOR PULLOUT FROM MOMENT ABOUT SIDE OF BRACKET:

Moment Mside = 0.78 * Fw_lat_out * (D/2 + 15") + 0.78 * Fw_lat_out * (M4/2 + 1.5")
= 14.455 kip*in

Wall Anchor Pullout Capacity = 2400 lbs > Mside / M4 = 14.455 kip*in / 24.41" = 592 lbs
Resistance to Wall Anchor Pullout Checks OK

CHECK ANCHOR PULLOUT FROM COMBINED MOMENTS:

Mbot / [2 anchors * (24" + 2") * R_pullout] + Mside / (1 anchor * M4 * R_pullout) = 0.546 < 1.00
Resistance to Wall Anchor Pullout Checks OK

CHECK MOMENT AND BOLT TENSION OF BRACKET BOLT:

Moment Mot = 0.78 * Fw_lat_out * H/2 + (0.78 * Fw_vert - 0.67 * Wt) * M4 / 2
= 18.555 kip*in

Bolt Tension Capacity = 1104lbs > Mot / (2 bolts * M1) = 18.555 kip*in / (2 * 14.38") = 645 lbs
Resistance to Overturning Checks OK

CHECK DIAGONAL BRACING:

Mside / 1.5" = 14.455 kip*in / 1.5" = 9637 lbs > R_pullout = 2400 lbs
Therefore, horizontal diagonal bracing is required at the top angles of the bracket.
Use X-brace with 1/8" x 1.5" bars.

Tension capacity R_bar = 1.5" * 0.125" * 36 ksi / Ω
= 1.5" * 0.125" * 36 ksi / 1.67
= 4.04 kip

Bar Tension = 0.6 * Fw_lat_out * (M1/2 + 15") / (M1 + 15") * √ [M4² + (M1 + 15")²] / M4
= .216 kip < 4.04 kip

Diagonal Bracing Resistance to Tension Checks OK

CHECK SHEET METAL ENVELOPE FASTENER RESISTANCE:

Analysis based on AISI S100-2007 "Cold Formed Steel Structural Members" Section E4:Screw Connections

Use Load Combination FBC 1605.3.1 Eq. 16-15

0.6D + 0.6W

On long side worst case

0.60 * Fw_lat_out = 0.60 * 875# = 525 lbs

Resistance to the metal shell pull-off is the minimum of the tensile strength of the screw and the pull-over strength of the sheet metal.

Inputs:

#10 screw, d = 0.19" with integral 0.5"-diameter washer

Thickness of metal shell, t1 = 0.043" (18 gauge)

Depth of penetration of screw into frame, tc = 0.25"

Strength of screw, Fu1 = 55 ksi

Based on the above data:

Allowable tensile of the screw, Pts/Ω = 321 lbs per screw (where Ω = 3.0)

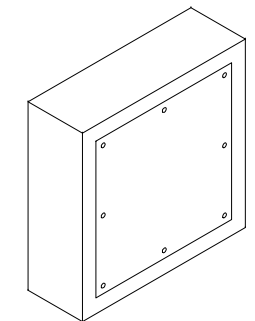
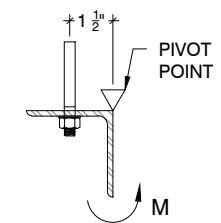
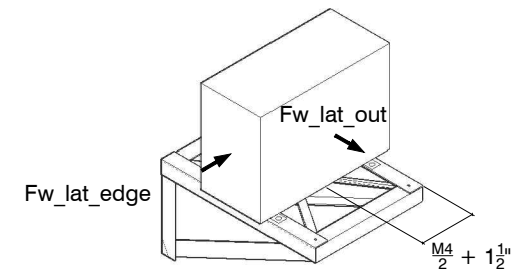
Allowable Pull-out strength, Pnot/Ω = 170 lbs per screw

Allowable Pull-over strength, Pnov/Ω = 371 lbs per screw

Therefore the min number of screws per long side = 525# / 170#/screw = 3.09 screws

Rounds up to min 4 screws per side, use 8 screws for symmetry and spacing.

Anchor Resistance to Metal Enclosure Pull-Off Checks OK.



SCREW PATTERN

SCALE: NTS



REVISIONS	
NO.	DESCRIPTION

SCALE	DATE
NTS	11/11/16
DRAWN BY	PROJECT MGR
JDP	PCP
PROJECT NO.	FLAT FILE
160387	
DRAWING NO.	
37-13-W-114	
Z4	
SHEET 2 OF 2	